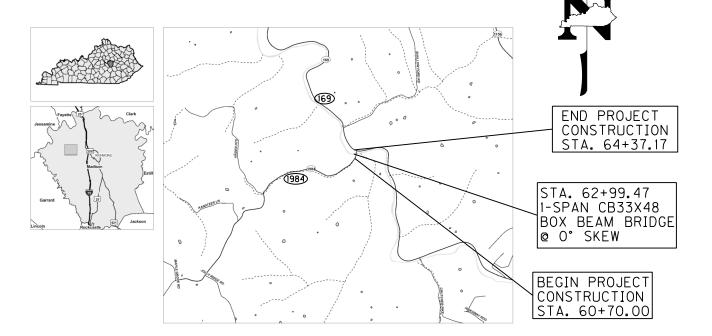


TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

MADISON COUNTY
KY 1984 OVER TATES CREEK
STA. 62+99.47



BRIDGING KENTUCKY

LOCATION MAP

| BEFORE YOU | | |
|---|--|------|
| The contractor is instructed to call 1-800-752-6007 to rea for information on the location of existing underground u a minimum of two (2) and no more than ten (10) business contractor should be aware that owners of underground members of the KY 811 one-call Before-U-Dig (BUD) servi excavation with the utility owners, including those whom may be necessary for the contractor to contact the Count utility companies have facilities in the area. | tilities. The call is to be placed s days prior to excavation. The facilities are not required to be ce. The contractor must coordinate do not subscribe to KY 811. It | |
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| REV. NO. | SHEETS REVISED | DATE |
| | TABLE OF REVISIONS | |

ROADWAY
SHEETS RI-R4

STRUCTURE
SHEETS SI-S6

OF KENTONIA

| COUNTY OF | ITEM NO. | SHEET NO. |
|-----------|----------|-----------|
| MADISON | 7-10001 | RI |

| INDEX OF SHEETS | | | | | | | | | |
|-----------------|--|--|--|--|--|--|--|--|--|
| Sheet No. | Description | | | | | | | | |
| Ri | LAYOUT SHEET | | | | | | | | |
| R2 | TYPICAL SECTIONS | | | | | | | | |
| R3 | PLAN SHEET | | | | | | | | |
| R4 | PROFILE SHEET | | | | | | | | |
| | | | | | | | | | |
| S01 | GENERAL NOTES | | | | | | | | |
| S02 | LAYOUT | | | | | | | | |
| S03 | FOUNDATION LAYOUT | | | | | | | | |
| S04 | END BENT DETAILS | | | | | | | | |
| S05 | SUPERSTRUCTURE DETAILS | | | | | | | | |
| S06 | CONSTRUCTION ELEVATIONS | | | | | | | | |
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| | SPECIAL NOTES | | | | | | | | |
| BRIDGE | OVERLAY APPROACH PAVEMENT | | | | | | | | |
| | TE SEALING | | | | | | | | |
| | N PREVENTION AND SEDIMENT CONTROL | | | | | | | | |
| | CONTROL ON BRIDGE REPAIR CONTRACTS | | | | | | | | |
| STRUCT | URES WITH OVER THE SIDE DRAINAGE | | | | | | | | |
| CONTRA | CT COMPLETION DATE AND LIQUIDATED DAMAGES ON | | | | | | | | |
| | IDGE REPAIR CONTRACTS | | | | | | | | |
| | LEARING RESTRICTION | | | | | | | | |
| BKY STI | | | | | | | | | |
| ADDITIO | NAL ENVIRONMENTAL COMMITMENTS | | | | | | | | |
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SPECIFICATIONS

SPECIAL PROVISIONS

69 - EMBANKMENT AT END-BENT STRUCTURES

Standard Specifications for Road and Bridge Construction, Current Edition.

2017 AASHTO LRFD Bridge Design Specifications with Current Interims.

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS COUNTY OF

MADISON

| NUMBER: | 24, 2020 | | |
|-------------------|------------------------|-------|--|
| RECOMMENDED BY: | | | |
| THE COMMENDED BY: | PROJECT MANAGER | DATE: | |
| PLAN APPROVED BY: | STATE HIGHWAY ENGINEER | DATE: | |

DESIGN CRITERIA CLASS OF HIGHWAY RURAL MINOR COLLECTOR TYPE OF TERRAIN

TYPE OF TERRAIN

DESIGN SPEED

REQUIRED NPSD

REQUIRED PSD

LEVEL OF SERVICE

ADT PRESENT (2016) 366

ADT FUTURE ()

DHY

GEOGRAPHIC COORDINATES

LATITUDE 37 DEGREES 46 MINUTES 45 SECONDS NORTH LONGITUDE 84 DEGREES 23 MINUTES 11 SECONDS WEST

DESIGNED

EXISTING BRIDGE ID 076B00071N

AECOM

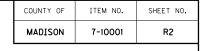
CONVENTIONAL SIGNS SURVEY LINE GRADE LINE GROUND LINE COUNTY LINE CORPORATE LIMITS EXIST. PROPERTY LINE EXIST. RIGHT OF WAY & PROPERTY LINE PROPOSED RIGHT OF WAY RIGHT OF WAY MONUMENT RENCH MARK B.M. NO.4 EXISTING R/W MARKER RIGHT OF WAY MONUMENT 0 UTILITY TEST HOLE EXISTING ROAD FENCE (CONTROLLED ACCESS) FENCE (EXCEPT STONE AND HEDGE) TREE LINE TREES PIPE CULVERT CULVERT BRIDGE BUILDINGS GUARDRAIL Z TERMINAL SECTION GUARDRAIL EXISTING PROPOSED а LIGHTING POLE \Box POWER POLE ŏ JOINT POWER & TELEPHONE POLE TELEPHONE & TELEGRAPH POLE Þ ANCHOR, POWER OR TELEPHONE 占 STUB POWER ò STUB TELEPHONE WATER MAIN — — wм — — **→ йм ⊢** GAS MAIN TELEPHONE DUCT ELECTRIC DUCT DIRECT BURIAL TV CABLE = = = SAN = SAN \longrightarrow SANITARY SEWER (WITH MANHOLE) STORM SEWER (WITH MANHOLE) DIRECT BURIAL ELECTRIC CABLE DIRECT BURIAL TELEPHONE CABLE OVERHEAD WIRE TRAFFIC LIGHTS ELECTRIC MANHOLE (FMH) (FMH) TELEPHONE MANHOLE (TMH) STONE FENCE _____ HEDGE FENCE SWAMP OR MARSH <u>M</u> <u>M</u> <u>M</u> <u>M</u> <u>M</u> <u>M</u> <u>M</u> SPRINGS PAPABAABAAB SINKHOLE QUARRY SITE BLUE LINE STREAM INTERMITTENT STREAM OR DITCH

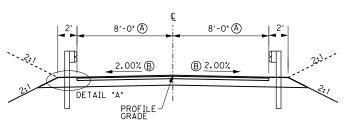
LAKES OR PONDS

NORTH POINT

REGULATED FLOODWAY

TYPICAL SECTIONS KY 1984 (MAPLE GROVE ROAD)





ROADWAY NORMAL SECTION FULL DEPTH CONSTRUCTION

STA. 60 + 70.00 TO STA. 62 + 58.47 STA. 63 + 40.47 TO STA. 64 + 37.17

VARIABLE DIMENSIONS AND SLOPES

- (A) LANE WIDTHS WIDEN FOR RADIUS RETURNS AT INTERSECTION WITH KY 169 (TATES CREEK ROAD).
- (B) LANE CROSS SLOPES VARY TO MATCH EXISTING AT TIE IN WITH EXISTING KY 1984 (MAPLE GROVE ROAD). LANE CROSS SLOPES VARY TO MATCH PAVEMENT EDGE GRADE AT INTERSECTION WITH KY 169 (TATES CREEK ROAD).

FULL-DEPTH MAINLINE & SHOULDER PAVEMENT RECONSTRUCTION

TRAFFIC LANES:

DENSE GRADED AGGREGATE 8* DEPTH
CL2 ASPH BASE 1.00D PG64-22 4* DEPTH
CL2 ASPH BASE 1.00D PG64-22 4* DEPTH
CL2 ASPH SURF 0.38D PG64-22 1.25* DEPTH

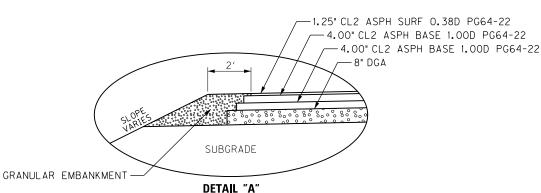
SHOULDERS:

DENSE GRADED AGGREGATE 8 DEPTH
CL2 ASPH BASE 1.00D PG64-22 4 DEPTH
CL2 ASPH BASE 1.00D PG64-22 4 DEPTH
CL2 ASPH SURF 0.38D PG64-22 1.25 DEPTH

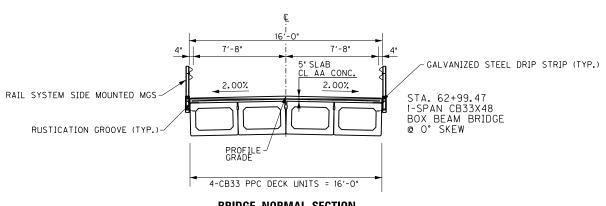
NOTES:

DGA BASE OR OTHER GRANULAR MATERIAL APPROVED BY THE ENGINEER NEEDED FOR SHOULDERS OUTSIDE OF PAVED AREA WILL BE MEASURED AND PAID AS GRANULAR EMBANKMENT IN ACCORDANCE WITH THE SPECIAL NOTE FOR BRIDGE OVERLAY APPROACH PAVEMENT.

GRANULAR EMBANKMENT UTILIZED AS FILL: THE SIDE SLOPES SHALL BE CAPPED WITH A MINIMUM OF 6 INCHES OF TOPSOIL STRIPPED FROM THE PROJECT SITE DISTURB LIMITS. THIS APPLICATION SHALL BE INCIDENTAL TO THE PLACEMENT OF GRANULAR EMBANKMENT. EMBANKMENT FOUNDATION BENCHING SHALL BE INCIDENTAL TO PLACEMENT OF GRANULAR EMBANKMENT.



FULL-DEPTH MAINLINE & SHOULDER
PAVEMENT RECONSTRUCTION
NOT TO SCALE



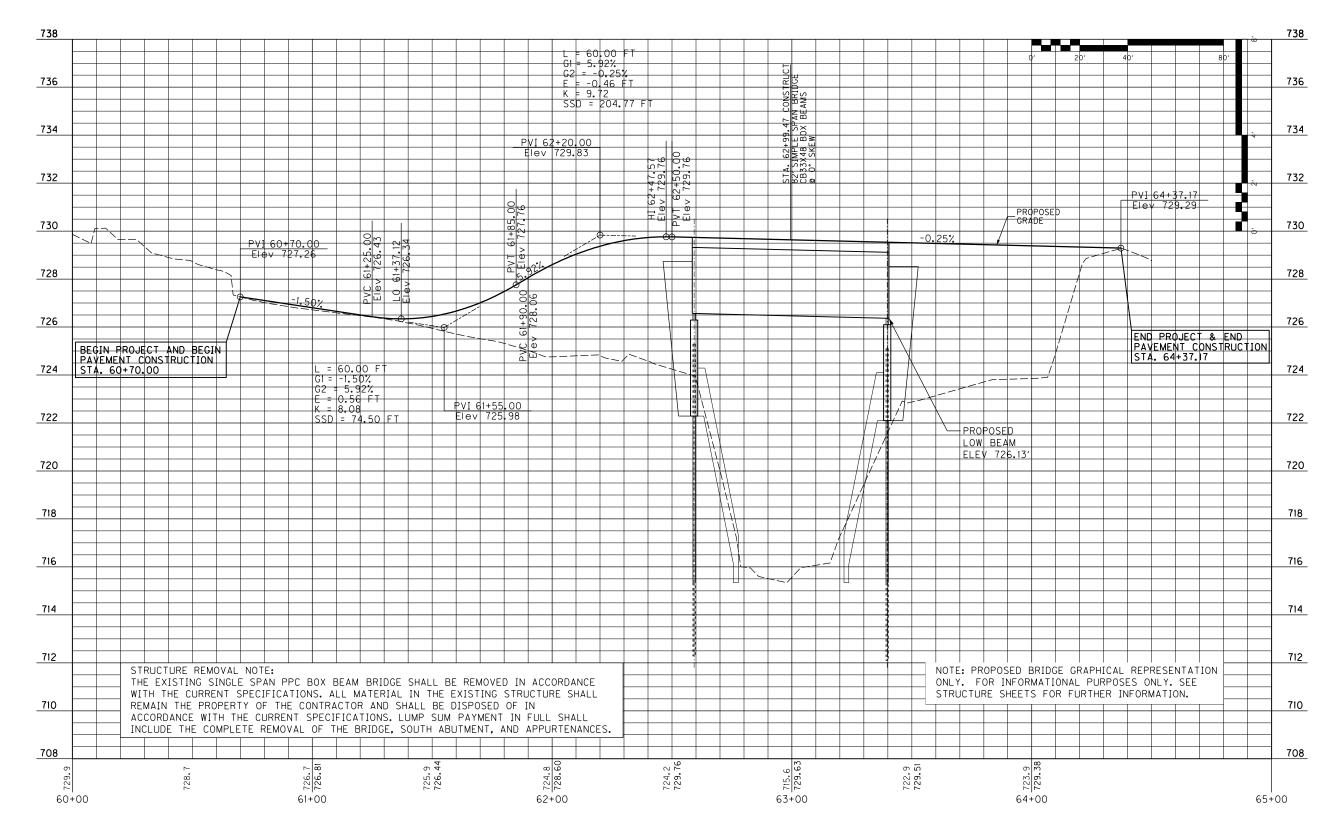
BRIDGE NORMAL SECTION
KY 1984 (MAPLE GROVE ROAD)
STA. 62+58.47 TO STA. 63+40.47





KY 1984 (MAPLE GROVE ROAD)
TYPICAL SECTIONS

| | 7–10001 KY–1984 (N | MADIE CDOS | TE ROAD) OV | TR TATES | CREEK | | | | | | | | | | | | | COUNTY | OF ITEM 1 | NO. SHEET NO. |
|---|--|---|---|---|---|--|---------------------------|---|---|--|--|---|---------------------|--|-------------|--|--|---|--|--|
| | /-10001 KY-1984 (F | | #076B00071N | EK TATES | CREEK | | | | | | | | | | | | | MADIS | | |
| | COORDIN | NATE C | ONTRO | L POI | NTS | | | | | | | | | FASE | MENTS | | | CEWER | | |
| | | T | Plane Coord | | | | PARCE | L 0000ED(0) | AREA OBTAINED | І т | AREA OF RACT | | ANENT R/W QUIRED | | TEMPORARY | PORTION REMAINING | SEWER | SEWER SYSTEM AFFECTED BY PROJECT | BUILDINGS ACQUIRED NUMBER | SOURCE OF |
| POINT | DESCRIPTION | | 1 | ELEV. (Z) | STATION | OFFSET | NO. | | | ACRES | SQ. FT. | ACRES | SQ FT | SQ. FT. | SQ. FT. | ACRES SQ. FT. | TYPE | | RFS | TITLE |
| CP 101 | 5/8" REBAR & CAP | 3810314.57 | 5315259.46 | 728.93 | 64+23.90 | 5.74′ RT | 1 | CHRISTOPHER CHRISTMAN & ASHLEY CHRISTMAN | PVA | 36.796 | | 0.003 | 128.50 | | | 36.793 | 1 | X | | DB 691 PG 795 |
| CP 102 | 5/8" REBAR & CAP | 3810267.50 | 5315389.04 | 728.25 | 63+72.41 | 141.85′ RT | 2 | LAMBERTO VARGAS & DONNA J. VARGAS | PVA | N/A | | 0.052 | 2266.95 | | | N/A | 5 | | | N/A |
| CP 103 | 5/8" REBAR & CAP | 3810176.76 | 5315360.76 | 722.27 | 62+82.07 | 112.29' RT | | DOUGLAS ROBERTS, ALLEN ROBERTS & | PVA | 2 | | | 13,227.22 | | | 1.70 | 1 | X | | DB 705 PG 510 |
| PROJECT CO | RENDA ROBERTS | | | | | | | | | | | | | | | | | | | |
| utilizing the K Coordinates, US BASIS OF ELI Elevations were NAVD88 vertice | rs on the NAD83 Kentucky St. SYCORS RTN GPS Network S Survey Feet. No project data EVATIONS e established by redundant GF cal datum, GEOID12B utilizing osed differential level loop bas | on January 28, 2 um factor was c PS observations u | 2019. Coordinates alculated or used using Carlson BR | shown are Stall for this projects Rx6 GNSS receive January 28. | ate Plane ect. | | | | De | O Nor D | ISTURB | | | | 3 // | 50 NOT DESC | | | 2. PRIVA 3. PUBLI 4. NONE | |
| BEGIN PR | ROJECT & BEGIN | P P P P P P P P P P P P P P P P P P P | 23.00 *43.16 (3.57) *11.25 51.1W | PROP R/W ———————————————————————————————————— | LAMBER & DONN 25:006+19 14 17 18 18 18 18 18 18 18 18 18 18 18 18 18 | PROF | DIUS = 302 ORD = 67.1 | DOUGLAS ROBERTS ALLEN ROBERTS & RENDA R STA. 62+99.47 CONSTRUCT 82'SIMPLE SPAN BRIDGE CB33X48 BOX BEAMS 0'SKEW 148.33 PROP R/W 148.33 PROP R/W | 3+76,25 | PI= R=1 DEI L=5 e=1 T=2 | 1984 #2 64+04.2 50.00 TA=21°0 55.27' NC 27.95' 2.58'.22.00 31.49 | 0 6'46" R | 1 23/15/2 | 05,450 | | CONSTRU LT STA. CONSTRU 37.5 LF CONNECT LT STA. CONSTRU LT STA. | CT SNOW 61+69 TO CT 1-GUAR GUARDRAII TO RAIL 62+58 TO CT 82 LF 63+40 TC TO RAIL CT 12.5 LI 6AIL END | STA. 63+4 BRIDGE RAI STA. 63+9 BRIDGE RAI STA. 63+9 SYSTEM SII F GUARDRAI TREATMENT | LF) TREATMENT T BEAM-S FACE BE MOUNTED N C L SYSTEM SI T DE MOUNTED N -STEEL W BE TYPE 7 | MGS DE MOUNTED MO MGS EAM-S FACE |
| STA. 60- N 38099 E 531520 | +70.00 73.49 07.45 RADIUS = 3 RADIUS = 3 RADIUS = 3 | | PI 61+45,09 | CHORD 1. 174.85 67.3 3.83' ARC 35, 79 - 3' 512' 12' 22' W | EX R/W DOO | 0° 48′41′ W 10.45 0.57 0.57 0.77 0.87 0.87 | 984 (MAF | RADIUS CHORD 14.50 RT STA. 63+49 REMOVE EXIST SIGN (WEIGHT LIMIT 6 TONS) LIMIT 510- 13. CLE GROVE ROAD) | = 629,36′ A = 68,47′ S69 35′53′W | RC 68.50 - | © 50'R TYP. C. P. | 0 0 0 | 18:05. E | | <i>y</i> | CONNECT RT STA. CONSTRU RT STA. CONNECT CONSTRU 1-GUARDR RT STA. REMOVE | TO RAIL 62+58 TO CT 82 LF 63+40 TO TO RAIL CT 25 LF RAIL END 62+29 TO | STA. 63+4 BRIDGE RAI STA. 63+7 SYSTEM SII | DE MOUNTED M O L SYSTEM SII 9 DE MOUNTED M STEEL W BEA TYPE 7 | DE MOUNTED MO |
| POINT POB PC PI PT PC PI | State Plane Coordinat NORTH (Y) EAST (X 3809916.11 5315167.3 3809995.36 5315222.7 3810035.03 5315250.4 3810083.42 5315249.7 3810269.33 5315247.1 3810297.28 5315246.7 | s STATION 4 60+00.00 4 60+96.69 6 61+45.09 8 61+90.32 4 63+76.25 5 64+04.20 | KY 1984 PI=61+45. R=150.00 DELTA=35 L=93.63' e=NC T=48.40' E=7.61' | #1 RT RE RE (WI | DO STA. 62+50 MOVE EXIST SIGN EIGHT LIMIT 6 TON | NOT DISTURE S) EXIST SINGLE SPANAND SOUTH ABUTME | Weight Limit 6 Tons | EXISTING NORTH ABUTMENT DO NOT DISTURB PAV Building TRUSTEES OF BIBLE MISS AVERY MASTE AVERY MASTE | MOVE EMENT THE MILLION, ROGER I ON, ROGER I RS, WALLACE | MAPLE G | ELEV | BHEND CLOCK ROOD 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | EX B./W | PAV STA N 3 E 5: RT KY 169 REMOVE & (STOP) | | 79 | ASONABLE ALL BE M. E PROJECT IST ALSO | AINTAINED 1 | INGRESS AND O ALL PROPE CCESS TO FIF NED AT ALL |) EGRESS RTIES WITHIN RE HYDRANTS TIMES. 80' |
| P T P O E | 3810323.50 5315256.4 3810340.83 5315262.8 | | | | • PAVEMENT TO BE SHALL BE INCIDENTA | | | DED. SEEDING RICHARDSON, AND | PAUL HOW | ARD | / ! | KY 16 | A | PREPARED BY | BRIKEN | DGING TUCKY | | PLAN | E GROVE SHEET TO STA.6 | |





<u>Design Load</u>: This bridge is designed for KYHL-93 live load, (i.e. 1.25xAASHTO HL93 live load). This bridge is designed for a future wearing surface of 15 psf.

<u>Design Method</u>: All reinforced concrete members are designed to be equivalent or greater than the load and resistance factor design method as specified in the current AASHTO Specifications.

Materials Design Specifications:

For Class "A" Reinforced Concrete f'c = 3500 psi For Class "AA" Reinforced Concrete f'c = 4000 psi For Steel Reinforcement fy = 60000 psi

<u>Material Specifications:</u> AASHTO Specifications or ASTM, current edition, as designated below shall govern the materials furnished.

AASHTO MI53 Premolded Cork Filler, Type II

AASHTO M-31 Deformed and Plain Billet-Steel for Concrete Reinforcement,

Grade 60

<u>Preformed Cork Expansion Joint Material</u>: Preformed Cork Expansion Joint Material shall conform to subsection 807.04.02 (Type II) of the Kentucky Department of Highways Standard Specifications.

<u>Concrete</u>: Class "AA" Concrete is to be used throughout the superstructure and in the portions of the substructure above the tops of caps. Class "A" concrete is to be used in the substructure below the caps. Prestressed beam concrete shall be in accordance with the plans and specifications

Reinforcement: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Any reinforcing bars designated by suffix "e" in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix "s" in a Bill of Reinforcement shall be considered a stirrup for purposes of bend diameters.

<u>Construction Identification</u>: The names of the Prime Contractor and the Sub-Contractor shall be imprinted in the concrete with Linch letters at a location designated by the engineer. The contractor shall furnish all plans, equipment and labor necessary to do the work for which no direct payment will be made.

Beveled Edges: All exposed edges shall be beveled $\frac{3}{4}$, unless otherwise shown.

<u>Payment for Precast Concrete Beams:</u> The basis of payment for the Prestressed Concrete Beams shall be at the contract unit price per linear foot of beam, in accordance with the specifications.

<u>Slope Protection</u>: Slope Protection at abutments shall be dry cyclopean stone riprap in accordance with the plans and specifications. Geotextile Fabric, Class I shall be placed between the embankment and the slope protection in accordance with Standard Specifications 214 and 843. Payment for Geotextile Fabric, Class I, shall be considered incidental to the unit price bid for Dry Cyclopean Stone Riprap.

<u>Completion of the Structure</u>: The contractor is required to complete the structure in accordance with the plans and specifications. Material, labor, or construction operations not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor, or anything else required to complete the structure.

Shop Drawings: The fabricator shall submit all required shop plans, by email to SHOP_076B0007 IN@docs.e-Builder.net, for review. These submissions shall depict the shop plans in .PDF format, as either Il*xI7* or 22*x36" sheets. Designers will make review comments on these electronic submissions as needed and, if required, shall return them to the fabricator for corrections and resubmittal. Upon acceptable reconciliation of all comments, files shall be sent to the Bridging Kentucky Shop Plan Coordinator for distribution. Only plans submitted directly to the Shop Plan Coordinator will be distributed. Additionally, only plans electronically stamped "Distributed by The Bridging Kentucky Program Team" are to be used for fabrication. While this process does not require the submission of paper copies, the Engineer of Record reserves the right to require such copies on a case by case basis.

When any changes to the design plans are proposed, the shop drawings reflecting these changes shall be submitted through the process above.

Note: The designation in the email 076B0007IN refers to the Bridge ID number which is located on the Title Sheet, RI of the Bridge Plans. Example: SHOP_076B0007IN@docs.e-Builder.net

<u>Utilities:</u> The contractor shall be responsible for locating any and all existing utilities prior to excavation of material or installation of guardrail or other construction activities that may involve utilities (overhead or underground).

General Notes

<u>Verifying Field Conditions:</u> The contractor shall field verify all dimensions before ordering material. New material that is unsuitable because of variations in the existing structure shall be replaced at the contractor's expense.

<u>Dimensions</u>: Dimensions are for a normal temperature of 60 degrees fahrenheit. Layout dimensions are horizontal dimensions.

<u>Superstructure Slab</u>: The superstructure slab shall be poured continuously from end to end of slab before the concrete is allowed to set.

<u>Mastic Tape</u>: Mastic Tape used to seal joints is to meet the requirements of ASTM C-877 Type I, II, or III. The joint is to be covered with 12" wide mastic tape. Prior to application, the joint surface shall be clean and free of dirt, debris, or deleterious material. Primer, if required by the tape manufacturer, shall be applied for a minimum width of 9" on each side of the joint.

Mastic Tape shall be either:

EZ-Wrap Rubber by Press-seal Casket Corporation, Seal Wrap by Mar Mac Manufacturing Co. Inc., Cadilloc by The UP Rubber Co. Inc. or approved equal.

Mastic Tape shall cover the joint continuously unless otherwise shown in the plans. Mastic Tape shall be spliced by taping a minimum of 6" and in accordance with the manufacturer's recommendations with the overlap running downhill.

The cost of labor, materials, and incidental items for furnishing and installing Mastic Tape shall be considered incidental to the unit price bid for concrete class 'AA' and no separate measurement of payment shall be made.

 $\underline{\text{Temporary Supports}}$: Temporary Supports or shoring will not be permitted under the beams when pouring the concrete deck slab or when taking "top of beam" elevations.

<u>Armored Edge</u>: Fabricate armored edge to match cross slope and parabolic crown at each end of bridge.

<u>Elastomeric Bearing Pads</u>: Elastomeric Bearing Pads shall conform to the AASHTO Standard Specifications for Highway Bridges, Division II, Section 18.

Bearings shall be Low Temperature Grade 3 with a shear modulus between 95 psi and 130 psi and shall be subjected to the load testing requirements corresponding to Design Method B. The cost of bearing pads is to be included in the unit price per linear feet for Precast Reams.

<u>Foundation Preparation:</u> Foundation Preparation shall be in accordance with Section 603 of the Specifications.

Foundation excavations should be properly braced/shored to provide adequate safety to persons working in or around excavations. Bracing should be performed in accordance with applicable federal, state and local guidelines.

Temporary shoring, sheeting, cofferdams, and/or dewatering methods may be required to facilitate foundation construction. It should be anticipated that groundwater will be encountered at foundation locations within the flood plain.

Temporary shoring, bracing, sheeting, cofferdams and dewatering shall be included in the Lump Sum Bid for Foundation Preparation.

<u>Structural Granular Backfill:</u> Materials for Structural Granular Backfill shall be in accordance with Section 805 of the Specifications.

Contrary to the Specifications, Structural Granular Backfill will not be measured for payment but shall be included in the Lump Sum Bid for Foundation Preparation.

Concrete Sealer:

Apply concrete sealer in accordance with the Special Note Concrete Sealing.

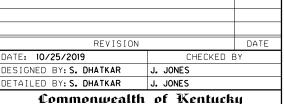
No Geotechnical Report: This Bridge did not have any drilling performed because rock was noted in the creek.

<u>Piling:</u> Piling shall be driven to practical refusal as defined on the pile record sheet.

Contrary to the standard drawings for steel piling, mill test reports are not required to be notarized.

<u>Pile Points:</u> Provide pile points for all piles. Pile points shall be in accordance with Section 604 of the specifications and of the type shown on the pile record sheet.

<u>Pre-drilling Piles</u>: Where pre-drilling is necessary for pile installation, holes shall be drilled into solid rock. Minimum distance between bottom of cap or pile bent and pile tip shall be 10′-0°. Backfill the holes with sand or pea gravel after the pile is placed in the hole. A temporary casing may be required to prevent collapse of the hole. If used, remove the casing as the hole is backfilled. Drive piles to refusal after backfill operations are complete. Include the cost of all materials, labor, and equipment needed to pre-drill, backfill the holes, and drive the piles to refusal in the price per linear foot for 'Pre-drilling for Piles'.



Commonwealth of Kentucky
DEPARTMENT OF HIGHWAYS

MADISON

KY 1984 TATES CREEK

GENERAL NOTES

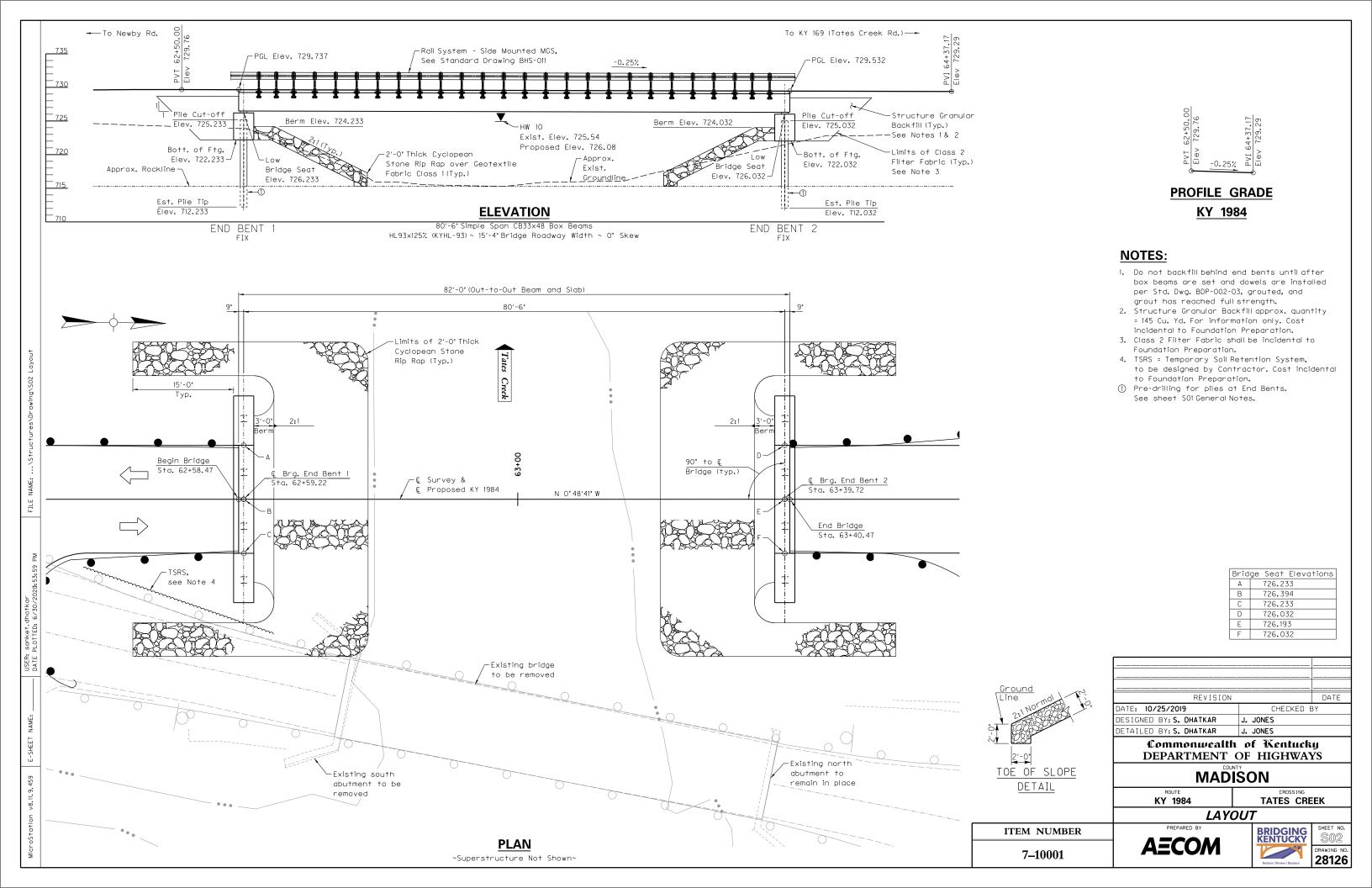
ITEM NUMBER

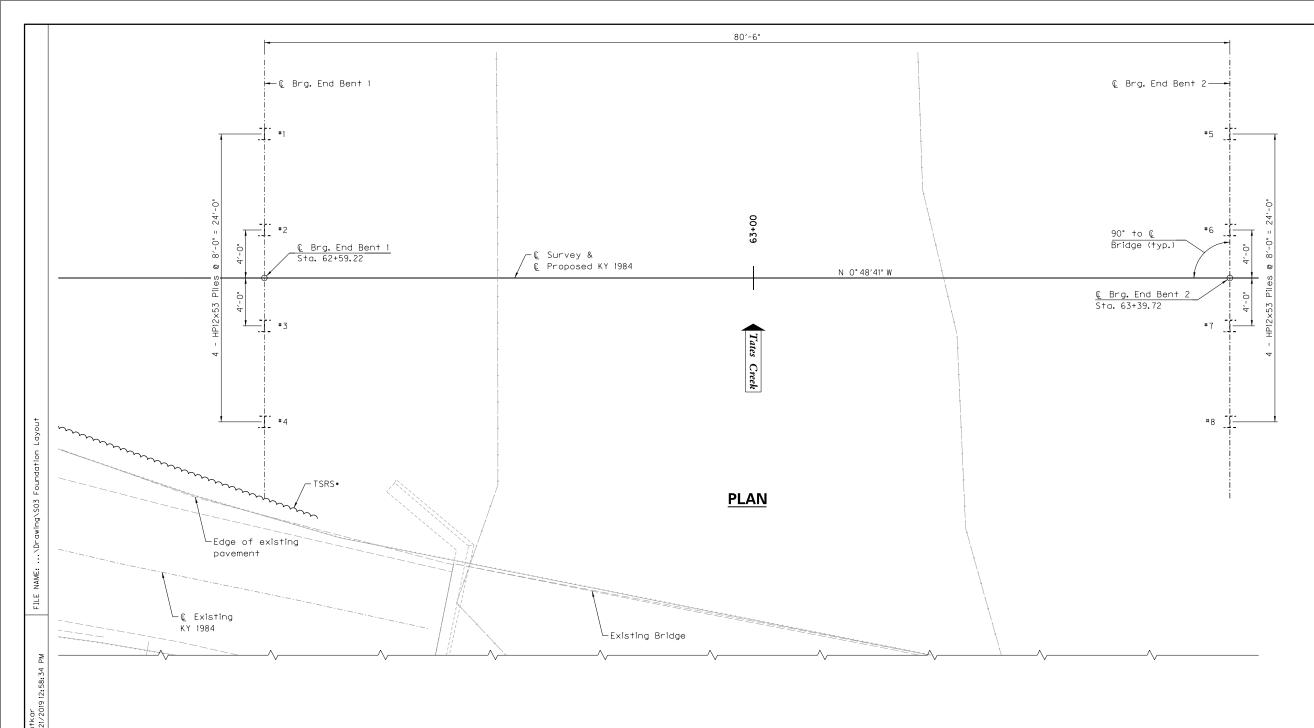
7-10001











TSRS = Temporary Soil Retention System,
 to be designed by Contractor. Cost incidental

to Foundation Preparation. Verify limits in field.

| F | PILE RECOP | <u>RD FOR PO</u> | <u>int bearin</u> | IG PILES |
|-------------|------------------------------|----------------------------|---|-------------------------|
| Pile No. | Pile Cut–off Elevation | Pile Length In Place | Point of Pile Elevation As Driven | Design Axial Load |
| | FEET | FEET | FEET | TONS |
| T | 725.233 | | | 93 |
| 2 | 725.233 | | | 93 |
| 3 | 725.233 | | | 93 |
| 4 | 725.233 | | | 93 |
| 5 | 725.032 | | | 93 |
| 6 | 725.032 | | | 93 |
| 7 | 725.032 | | | 93 |
| 8 | 725.032 | | | 93 |

Definitions of Terms

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure.
PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elevation in the finished

<code>DESIGN AXIAL LOAD: Load carried by each pile as estimated from structural design calculations for Factored LRFD Loadings.</code>

CALCULATED FIELD BEARING: Contrary to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to

Driving Criteria

DRIVING CRITERIA: Drive point bearing piles to practical refusal.

PRACTICAL REFUSAL (Case I): Drive point bearing piles to practical refusal. For this project minimum blow requirements are reached after total penetration becomes \(\frac{1}{4} \) or the soft on security blows, practical refusal is obtained after the pile is struck an additional 5 blows with total penetration of \(\frac{1}{4} \) inch or less. Advance the production piling to the driving resistances specified above and to the depths determined by test pile(s) and subsurface data sheet(s). Immediately cease driving operations if the pile visibly yields or becomes damaged during driving. If hard driving is encountered because of dense strata or an obstruction, such as a boulder before the pile is advanced to the depth anticipated, the Engineer will determine if more blows than the average driving resistance specified for practical refusal is required to further advance the pile. Drive additional production and test piles if directed by the Engineer.

Field Data

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place and Point of Pile Elevation as Driven.

Submit this record to:

Kentucky Transportation Cabinet Director, Division of Structural Design 3rd Floor East 200 Mero Street Frankfort, KY 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit.

Use HP 12x53 in accordance with BPS-003, c.e.

Notes

- I. A diesel pile driving hammer with a rated energy between 10.5 foot-kips and 20.1 foot-kips will be required to drive 12x53 steel H-piles to practical refusal without encountering excessive blow counts or damaging the piles. The Contractor shall submit the proposed pile driving system to the Engineer for approval prior to the installation of the first pile. Approval of the pile driving system by the Engineer will be subject to satisfactory field performance of the pile driving procedures.
- 2. If hard driving is encountered because of dense strata or an obstruction, such as a boulder before the pile is advanced to the depth anticipated, the Engineer will determine if more blows than the average driving resistance specified for practical refusal is required to further advance the pile. Drive additional production piles if directed by the Engineer.
- 3. The installation of the pile foundations should conform to current AASHTO LRFD Bridge Design Specifications, and Section 604 of the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction.
- 4. The Kentucky Transportation Cabinet recommends that protective pile points be used on end bearing piles to allow for embedment into the top of bedrock. Use of reinforced pile points capable of penetrating boulders and hard layers which may be encountered is recommended. Installation of pile points should be in accordance with Section 604 of the Kentucky Standard Specifications for Road and Bridge Construction, current edition.

| REVISION | DATE | |
|-------------------------|------------|--|
| DATE: 10/25/2019 | 3 Y | |
| DESIGNED BY: S. DHATKAR | J. JONES | |
| DETAILED BY: S. DHATKAR | J. JONES | |
| Cammanusalth | af Wantuah | |

Commonwealth of Kentucky
DEPARTMENT OF HIGHWAYS

MADISON

KY 1984 TATES CREEK

FOUNDATION LAYOUT

ITEM NUMBER

7–10001

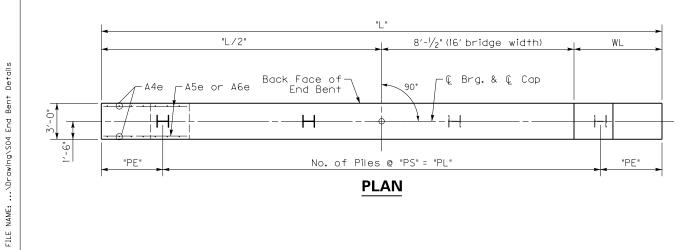
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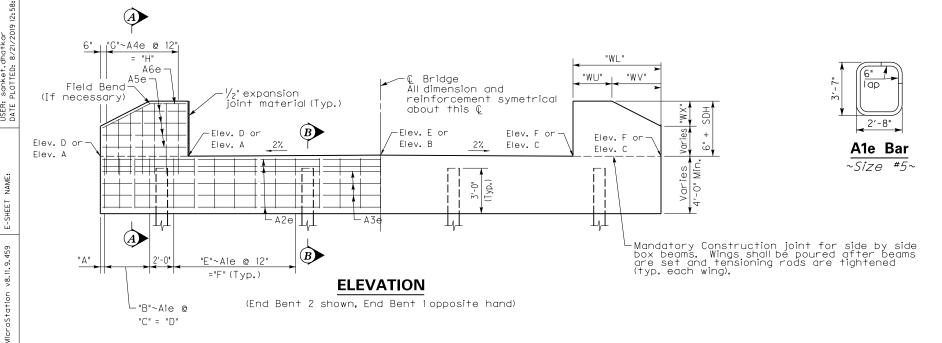




| SL | JPERSTRUC | CTUR | RE HEIGH | нT | CA | AP B | ILL | OF F | REI | NF (| ORC | EMEN | ΙT | | | G BI | | | | | | |
|--|-----------|-----------------|----------|---------------------|--------------------|------------------|-------|--------|---------------------------|------|-----|---------------|-----|-------|------|--------|-------|-------|--------|-------|--|--|
| SD +/bay | nd Heigi | n† | | 16'-0" BRIDGE WIDTH | | | | | | | | REINFORCEMENT | | | | | | | | | | |
| SDH = Beam+Pad Height +(haunch+slab)[if applicable] | | | | | MA | ARK | TYPE | NO | | SIZ | ĽE | LENGT | н | MARK | TYPE | NO. | SI | ZE LE | NGTH | | | |
| | | | | | А | le | I4s | 29 |) | 5 | | 13' -6' | 1 | А4е | Str. | 28 | Ę | 5 5' | - 9 '' | | | |
| Н3 | 35 | 35" < SDH ≤ 50" | | • | Α | 2e | Str. | 9 | | 8 | | 30′ -5 ' | · [| A5e | Str. | 16 | - 5 | 7' | -0" | | | |
| | | | | | Α | .3e | Str. | 6 | | 5 | | 30′ -5 ' | · [| A6e | Str. | 4 | (| 7′ | -5" | | | |
| BRID | GE PI | ILE I | LOAD | | | PILES DIMENSIONS | | | | | | | | | | | | | | | | |
| WIDT | TH SIZ | ZE | TONS | NO. | PE PS PL A B C D E | | | | F | G | Н | C | L | WL | WU | W∨ | WX | | | | | |
| 16 | нз | 3 | 93 | 4 | | 3'-41/2" | 8'-0" | 24'-0" | 24'-0" 41/2" 4 8" 2'-0" 7 | | | 6′-0" | 7 | 6'-0" | 4 | 30′-9" | 7'-4" | 3′-3" | 4'-1" | 2'-1" | | |

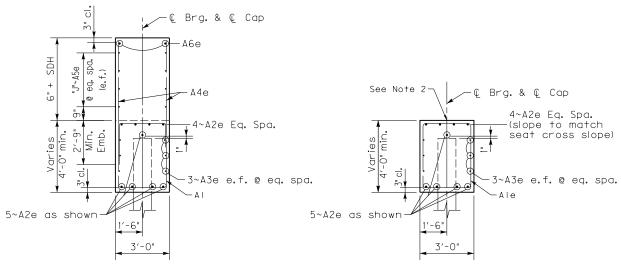
SDH = 40.69" for this sheet.





- 1) Conform to KYTC, Standard Specifications, Current Edition.
 2) Concrete to be Class "A", 3500 psi.
- 3) Rebar to be epoxy coated A615, Grade 60.
- 4) Maintain 2" clear cover to reinforcement unless otherwise noted.
- 5) End Bents are designed for the maximum span of the following concrete beams as shown in the current standards: H3 CB33.

- 8) Piles must be driven 10' into existing ground below pile cap.
 Pre drilling into rock will be required. Drive piles to refusal.
- 9) Contractor shall provide a hammer capable of driving the piling
- to refusal or capacity without encountering excessive blow counts or damaging the pile. Contractor shall be responsible for all damaged piling.



NOTES:

1. See Sheet SO2 for Bridge Seat Elevations. 2. Bridge Seat Elevations are provided at $\ensuremath{\mathbb{Q}}$ of

SECTION A-A

3. Slope cap as necessary with side by side box beams to maintain proper roadway slope.

7-10001

| REVISION | | DATE |
|-------------------------|-----------|------|
| DATE: 10/25/2019 | CHECKED E | 3 Y |
| DESIGNED BY: S. DHATKAR | J. JONES | |
| DETAILED BY: S. DHATKAR | J. JONES | |
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Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

MADISON

KY 1984 TATES CREEK **END BENT DETAILS**

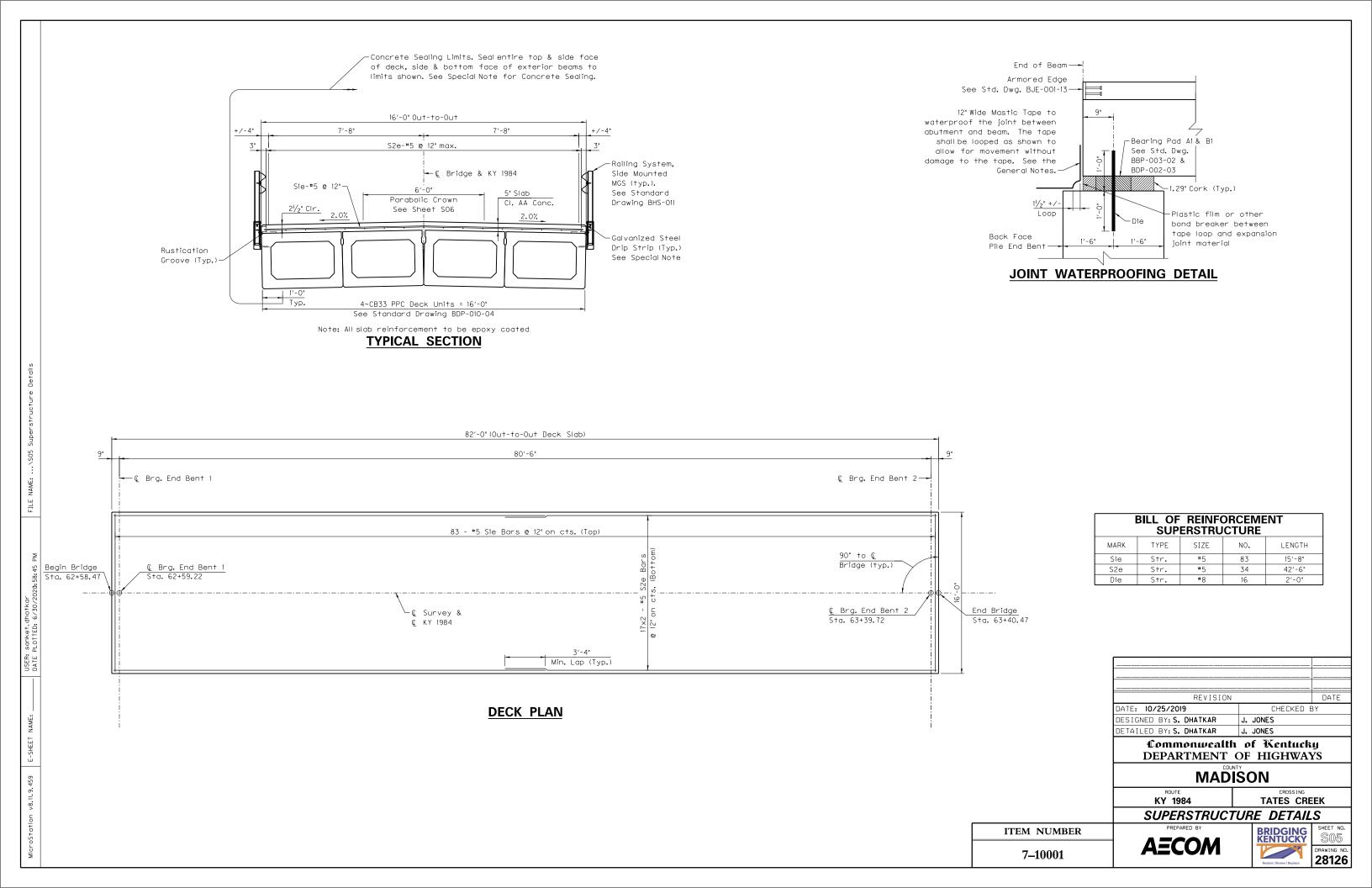
ITEM NUMBER

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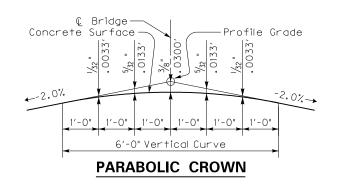
SECTION B-B

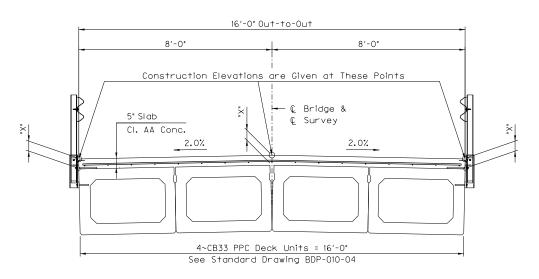




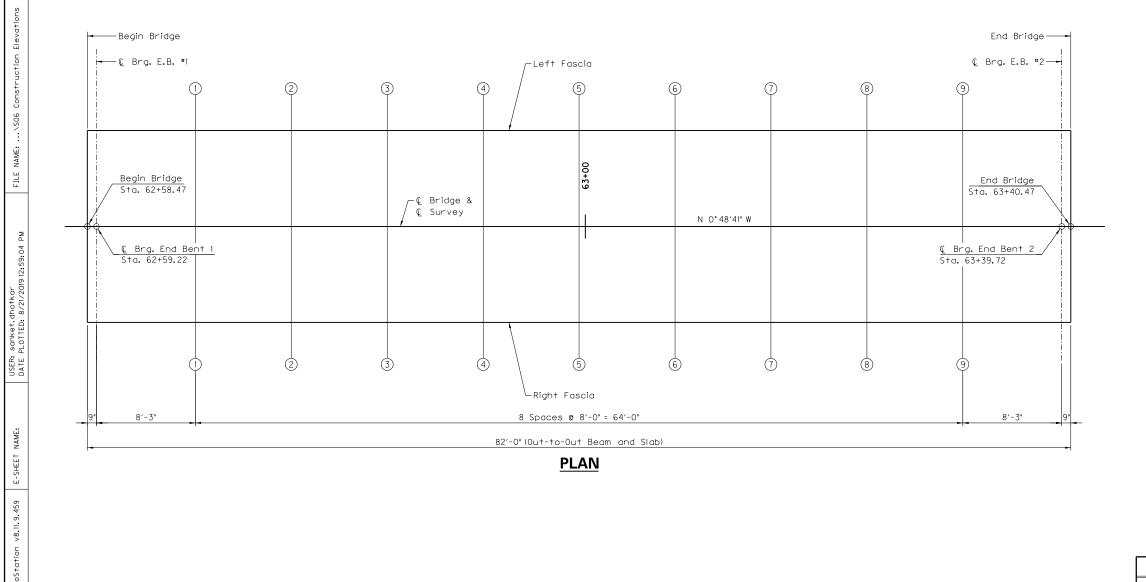


| | i | LEFT FASCIA | | CEN | TERLINE BRIG | OGE | F | RIGHT FASCIA | |
|----------------|-----------------|----------------|-------------|-----------------|----------------|-------------|-----------------|----------------|-------------|
| LOCATION | CONST. ELEV. | TOP OF BEAM | DIM. "X" | CONST. ELEV. | TOP OF BEAM | DIM. "X" | CONST. ELEV. | TOP OF BEAM | DIM. "X" |
| Begin Bridge | 729.577 | | | 729.737 | | | 729.577 | | |
| € Brg. E.B. #1 | 729.575 | | | 729.735 | | | 729.575 | | |
| 1 - 1 | 729.573 | | | 729.733 | | | 729.573 | | |
| 2 - 2 | 729.567 | | | 729.727 | | | 729.567 | | |
| 3 - 3 | 729.557 | | | 729.717 | | | 729.557 | | |
| 4 - 4 | 729.542 | | | 729.702 | | | 729.542 | | |
| 5 - 5 | 729.524 | | | 729.684 | | | 729.524 | | |
| 6 - 6 | 729.502 | | | 729.662 | | | 729.502 | | |
| 7 - 7 | 729.477 | | | 729.637 | | | 729.477 | | |
| 8 - 8 | 729.447 | | | 729.607 | | | 729.447 | | |
| 9 - 9 | 729.413 | | | 729.573 | | | 729.413 | | |
| © Brg. E.B. #2 | 729.374 | | | 729.534 | | | 729.374 | | |
| End Bridge | 729.372 | | | 729.532 | | | 729.372 | | |





TYPICAL SECTION



NOTES FOR ELEVATIONS TAKEN ON PRESTRESSED CONCRETE BOX BEAMS

Take elevations on top of beam at points indicated after the beams have been laterally tensioned and grouted. The beam elevations are to be read to three decimal places and entered in tables under "Top of Beam" elevations.

Compute dimension "X" as follows: "Construction Elevation" minus "Top of Beam" elevation equals dimension "X". Construction Elevations include camber due to weight of the concrete slab and barrier. Measuring of dimension "X" gives the final check on beam tolerances for camber, beam damage, and errors in erection that produce reverse cambers, sags, and unsightly fascia beams.

The minimum allowable dimension "X" or slab thickness is 5" (0.4167') If any computed dimension "X" is outside limits, adjustments need to be made to the dimensions "X" on one or more grid lines at the discretion of the Engineer.

For setting templates, measure dimension "X" above top of beams for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking "Top of Beam" elevations.

| REVISION | DATE | | | | | | | | | |
|---|------------|--|--|--|--|--|--|--|--|--|
| DATE: 10/25/2019 | CHECKED BY | | | | | | | | | |
| DESIGNED BY: S. DHATKAR | J. JONES | | | | | | | | | |
| DETAILED BY: S. DHATKAR | J. JONES | | | | | | | | | |
| Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS | | | | | | | | | | |

MADISON

KY 1984 TATES CREEK

CONSTRUCTION ELEVATIONS

ITEM NUMBER

7-10001



